## 12.520 Lecture Notes 27

## Flow in Porous Media

Problem of great economic importance (also scientific)

- hydrology (ground water migration, toxic waste)
- oil migration
- soil stability, fault mechanics (pore pressure)
- melt migration in mantle
- geysers and hot springs

Porous medium  $\Rightarrow$  voids  $\Rightarrow$  porosity  $\phi$ 

 $\phi \equiv$  volume fraction of voids

For example,

Sand:  $\phi \sim 40\%$ Pumice:  $\phi \sim 70\%$ Oil shales:  $\phi \sim 10-20\%$ If pore connected  $\Rightarrow$  permeable Pressure gradient  $\Rightarrow$  flow Darcy's law  $\Rightarrow \chi = -\frac{k}{\eta}\nabla p$  $\nu =$  volumetric flow rate k = permeability

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We can use Poiseuille flow for simple geometries. For example, cubical matrix, circular tubes or pipes.



Figure 27.1 Figure by MIT OCW.

$$\phi = \frac{12 \cdot \frac{1}{4} \cdot \pi \cdot \left(\frac{\delta}{2}\right)^2 \cdot b}{b^3} = \frac{3\pi}{4} \frac{\delta^2}{b^2}$$

Consider 
$$\frac{dp}{dx}$$
 (one direction only)  
In each pipe (along x),  $\bar{u} = -\frac{\delta^2}{32\eta} \frac{dp}{dx}$  [Poiseuille flow]  
Darcy velocity:  $v = \frac{4 \cdot \frac{1}{4} \cdot \bar{u} \cdot \pi \cdot \left(\frac{\delta}{2}\right)^2}{b^2} = \frac{\pi \delta^2}{4b^2} \bar{u} = \frac{\phi}{3} \bar{u}$   
 $v = -\frac{b^2 \phi^2}{72\pi\eta} \frac{dp}{dx}$   
 $\Rightarrow k = \frac{1}{72\pi} b^2 \phi^2$ 

Large 
$$b \Rightarrow$$
 large  $v$ ?  $b^2 = \frac{3\pi}{4} \frac{\delta^2}{\phi}$   
Large  $\phi \Rightarrow$  large  $v$ ?  $k = \frac{\pi}{128} \frac{\delta^4}{b^2}$ 

Compare to cubes separated along faces (channel flow)



Figure 27.2 Figure by MIT OCW.

$$\phi = \frac{6 \cdot \frac{1}{2} \cdot \delta b^2}{b^3} = 3\frac{\delta}{b}$$

Again, 
$$\frac{dp}{dx}$$
 directed along one edge  

$$u = \frac{1}{2\eta} \frac{dp}{dx} \left( Z^2 - (\delta/2)^2 \right)$$

$$\overline{u} = \frac{1}{2\eta\delta} \frac{dp}{dx} \left( \frac{Z^3}{3} - \frac{\delta^2 Z}{2} \right) \Big|_{-\delta/2}^{\delta/2} = -\frac{5\delta^2}{24\eta} \frac{dp}{dx}$$

Darcy velocity:  $v = 2\frac{b\delta}{b^2}\overline{u} = -\frac{5}{12}\frac{\delta^3}{b\eta}\frac{dp}{dx} = -\frac{5}{324}\frac{b^2\phi^3}{\eta}\frac{dp}{dx}$ 

$$k = \frac{5b^2\phi^3}{324}$$

*k* is different depending on  $\phi$ .

$$k = \frac{135}{324} \frac{\delta^3}{b}$$

Clearly, porosity distribution is important.



Figure 27.3 Figure by MIT OCW.

Also -- more easily measured than figured out theoretically – more complicated geometries  $\rightarrow$  numerical simulation.

Consider "Lawn Sprinkler" example – flow in unconfined aquifer.



Figure 27.4 Figure by MIT OCW.

$$h \equiv$$
 "hydraulic head"

 $u \rightarrow$  Darcy velocity

Dupuit approximation:  $\frac{dp}{dx} = \rho g \frac{\partial h}{\partial x}$ 

For  $\frac{\partial h}{\partial x} = 1$  flow is one-dimensional. Darcy's law:  $u = -\frac{k\rho g}{\eta} \frac{\partial h}{\partial x}$ 

Conservation of mass: Assume no input

Flux  $Q = u(x)h(x) = -\frac{k\rho g}{\eta}h\frac{dh}{dx} = \text{const.}$ 

 $\Rightarrow$  phreatic surface is a parabola

For  $h = h_0$  at x = 0

$$h = \left(h_0^2 - \frac{2Q\eta x}{k\rho g}\right)^{1/2}$$

Suppose we have a porous dam of width w. The relation between Q,  $h_0$  and  $h_1$  is:

$$Q = \frac{k\rho g}{2\eta w} \left( h_0^2 - h_1^2 \right)$$

or

$$Q = \frac{k\rho g}{2\eta w} \left[ \left( h_0 - h_1 \right) \left( h_0 + h_1 \right) \right]$$



Figure 27.5 Figure by MIT OCW.