

Lecture 6 - Finite Element Solution Process

In the last lecture, we used the principle of virtual displacements to obtain the following equations:

$$KU = R \tag{1}$$

$$K = \sum_m K^{(m)} \quad ; \quad K^{(m)} = \int_{V^{(m)}} B^{(m)T} C^{(m)} B^{(m)} dV^{(m)}$$

$$R = R_B + R_S$$

$$R_B = \sum_m R_B^{(m)} \quad ; \quad R_B^{(m)} = \int_{V^{(m)}} H^{(m)T} f^{B(m)} dV^{(m)}$$

$$R_S = \sum_m R_S^{(m)} \quad ; \quad R_S^{(m)} = \sum_i \int_{S_f^{i(m)}} H^{S_f^{i(m)T}} f^{S_f^{i(m)}} dS_f^{i(m)}$$

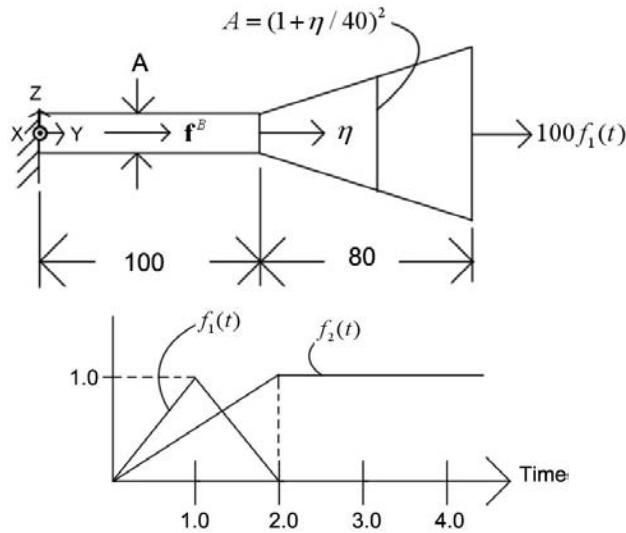
$$u^{(m)} = H^{(m)}U \tag{2}$$

$$\begin{matrix} \downarrow \\ \epsilon^{(m)} = B^{(m)}U \end{matrix} \tag{3}$$

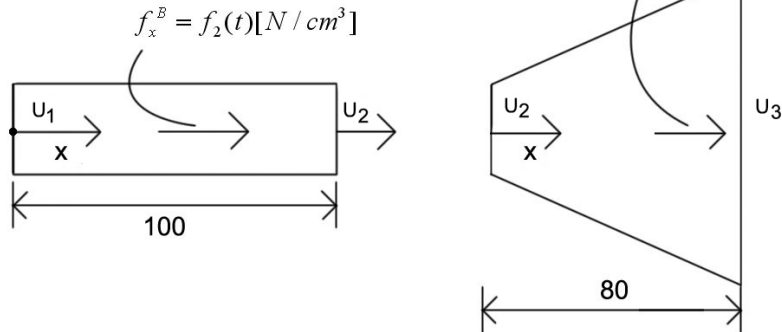
Note that the dimension of $u^{(m)}$ is in general not the same as the dimension of $\epsilon^{(m)}$.

Example: Static Analysis

Reading assignment: Example 4.5



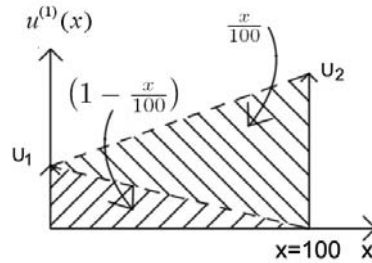
x is a local coordinate which is different from X . $f_x^B = 0.1f_2(t)[N/cm^3]$



Assume:

- i. Plane sections remain plane
- ii. Static analysis \rightarrow no vibrations/no transient response
- iii. One-dimensional problem; hence, only one degree of freedom per node

Elements 1 and 2 are compatible because they use the same U_2 . Next, use a linear interpolation function.

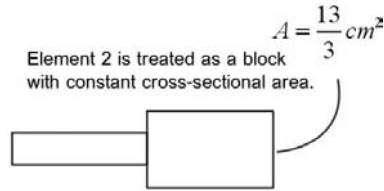


$$u^{(1)}(x) = \underbrace{\left[\left(1 - \frac{x}{100}\right) \quad \frac{x}{100} \quad 0 \right]}_{\mathbf{H}^{(1)}} \begin{bmatrix} U_1 \\ U_2 \\ U_3 \end{bmatrix} ; \quad u^{(2)}(x) = \underbrace{\left[0 \quad \left(1 - \frac{x}{80}\right) \quad \frac{x}{80} \right]}_{\mathbf{H}^{(2)}} \begin{bmatrix} U_1 \\ U_2 \\ U_3 \end{bmatrix}$$

$$\varepsilon^{(1)}(x) = \underbrace{\left[-\frac{1}{100} \quad \frac{1}{100} \quad 0 \right]}_{\mathbf{B}^{(1)}} \begin{bmatrix} U_1 \\ U_2 \\ U_3 \end{bmatrix} ; \quad \varepsilon^{(2)}(x) = \underbrace{\left[0 \quad -\frac{1}{80} \quad \frac{1}{80} \right]}_{\mathbf{B}^{(2)}} \begin{bmatrix} U_1 \\ U_2 \\ U_3 \end{bmatrix}$$

$$\begin{aligned} \mathbf{K} &= E \cdot 1 \cdot \int_0^{100} \begin{bmatrix} -\frac{1}{100} \\ \frac{1}{100} \\ 0 \end{bmatrix} \begin{bmatrix} -\frac{1}{100} & \frac{1}{100} & 0 \end{bmatrix} dx + E \int_0^{80} \left(1 + \frac{x}{40}\right)^2 \begin{bmatrix} 0 \\ -\frac{1}{80} \\ \frac{1}{80} \end{bmatrix} \begin{bmatrix} 0 & -\frac{1}{80} & \frac{1}{80} \end{bmatrix} dx \\ &= \frac{E}{100} \begin{bmatrix} 1 & -1 & 0 \\ -1 & 1 & 0 \\ 0 & 0 & 0 \end{bmatrix} + \frac{13E}{3 \cdot 80} \begin{bmatrix} 0 & 0 & 0 \\ 0 & 1 & -1 \\ 0 & -1 & 1 \end{bmatrix} \end{aligned}$$

The “equivalent cross-sectional area” of element 2 is $A = \frac{13}{3} \text{ cm}^2$. This equivalent area must lie between the areas of the end faces $A = 1$ and $A = 9$.



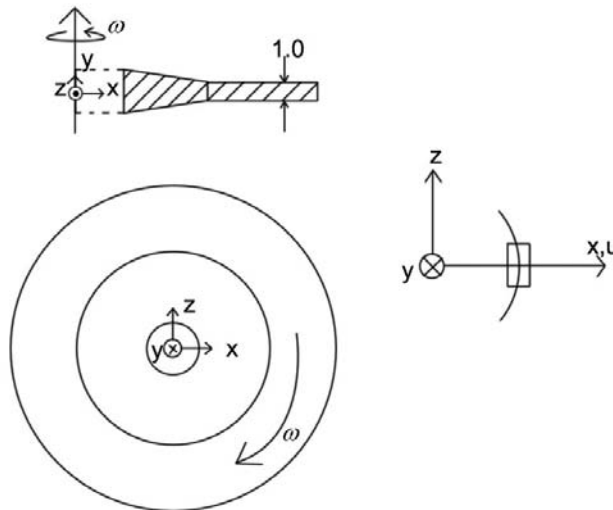
$$\mathbf{K} = \frac{E}{240} \begin{bmatrix} 2.4 & -2.4 & 0 \\ -2.4 & 15.4 & -13 \\ 0 & -13 & 13 \end{bmatrix}$$

We note:

- Diagonal terms **must** be positive. If the diagonal terms are zero or negative, then the system is unstable physically. A positive diagonal implies that the degree of freedom has stiffness at that node.
- \mathbf{K} is symmetric.
- \mathbf{K} is singular if rigid body motions are possible. To be able to solve the problem, all rigid body modes must be removed by adequately constraining the structure. i.e. \mathbf{K} is reduced by applying boundary conditions to the nodes.

The \mathbf{K} used to solve for \mathbf{U} is, then, positive definite ($\det \mathbf{K} > 0$). This ensures that the elastic strain energy is positive and nonzero for any displacement field \mathbf{U} . In the analysis, each element is in equilibrium under its nodal forces, and each node is in equilibrium when summing element forces and external loads.

Homework Problem 2



$$\begin{bmatrix} \epsilon_{xx} \\ \epsilon_{zz} \end{bmatrix} = \begin{bmatrix} \frac{\partial u}{\partial x} \\ \frac{u}{x} \end{bmatrix}$$

ε_{zz} is frequently called the “hoop strain”, $\varepsilon_{\theta\theta}$.

$$\varepsilon_{zz} = \frac{2\pi(u+x) - 2\pi x}{2\pi x} = \frac{u}{x}$$

$$\mathbf{C} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu \\ \nu & 1 \end{bmatrix}$$

$$\mathbf{f}^B = \rho\omega^2 R [N/cm^3] \quad ; \quad R = x$$

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